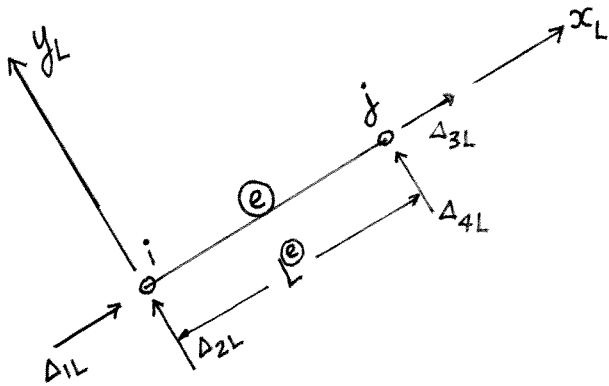
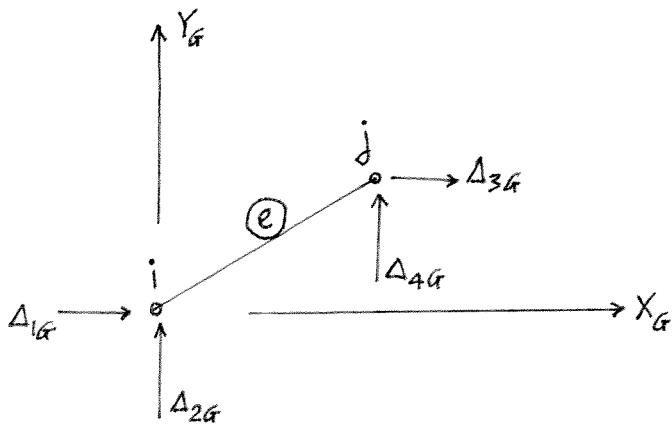


2-D Truss (2 dof per node, 4 dof per element)



$$k_L^e = \left(\frac{AE}{L^e} \right) \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$



$$k_G^e = [R]_{4 \times 4}^T [k_L^e]_{4 \times 4} [R]_{4 \times 4}$$

$$\text{where: } [R] = \begin{bmatrix} [C] & [0] \\ [0] & [C] \end{bmatrix}$$

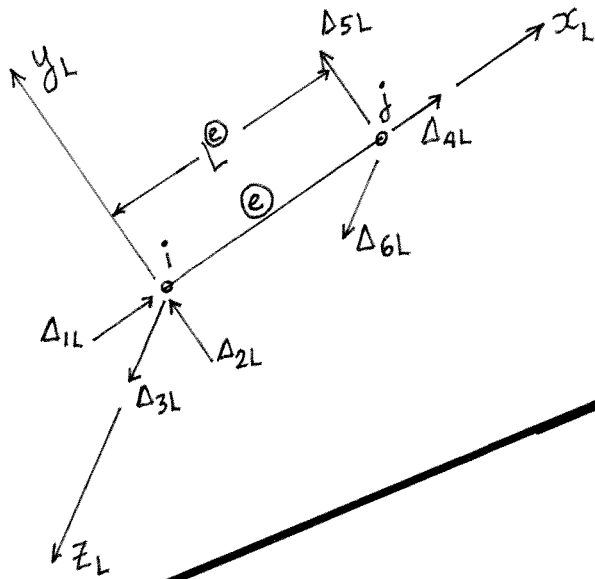
$$[C] = \begin{bmatrix} c_x & c_y \\ -c_y & c_x \end{bmatrix} \quad \begin{matrix} \nearrow \frac{y_j - y_i}{L^e} \\ \nwarrow \frac{x_j - x_i}{L^e} \end{matrix}$$

Hence:

$$[k_G^e] = \frac{EA}{L} \begin{bmatrix} C_x^2 & C_x C_y & -C_x^2 & -C_x C_y \\ C_x C_y & C_y^2 & -C_x C_y & -C_y^2 \\ -C_x^2 & -C_x C_y & C_x^2 & C_x C_y \\ -C_x C_y & -C_y^2 & C_x C_y & C_y^2 \end{bmatrix}$$

$$\text{where } L = L^e = \sqrt{(x_j - x_i)^2 + (y_j - y_i)^2}$$

3-D Truss (3 dof per node, 6 dof per element)



$$k_L^{(e)} = \left(\frac{AE}{L^{(e)}} \right) \begin{bmatrix} 1 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -1 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$k_G^{(e)} = [R]^T [k_L^{(e)}] [R]$$

6x6 6x6

where:

$$[R] = \begin{bmatrix} [C] & [0] \\ [0] & [C] \end{bmatrix}$$

$$[C]_{3 \times 3} = \begin{bmatrix} c_x & c_y & c_z \\ -\frac{c_x c_y}{c_{xz}} & c_{xz} & -\frac{c_y c_z}{c_{xz}} \\ -\frac{c_z}{c_{xz}} & 0 & \frac{c_x}{c_{xz}} \end{bmatrix}$$

$\frac{z_j - z_i}{L^{(e)}}$
 = General orientation member
 $\sqrt{c_x^2 + c_z^2}$

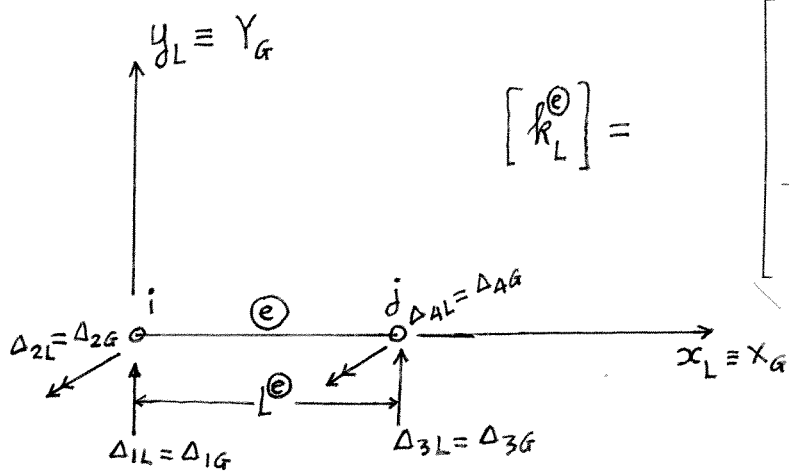
$$[C_{vertical}] = \begin{bmatrix} 0 & c_y & 0 \\ -c_y & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} = \text{vertical member (parallel to } Y_G \text{ axis)}$$

Hence:

$$[k_G^{(e)}] = \left(\frac{EA}{L} \right) \begin{bmatrix} c_y^2 & c_y c_x & c_y c_z & -c_y^2 & -c_y c_x & -c_y c_z \\ c_y c_x & c_x^2 & c_x c_z & -c_x c_y & -c_x^2 & -c_x c_z \\ c_y c_z & c_x c_z & c_z^2 & -c_y c_z & -c_y c_x & -c_z^2 \\ -c_y^2 & -c_y c_x & -c_y c_z & c_y^2 & c_y c_x & c_y c_z \\ -c_x c_y & -c_x^2 & -c_x c_z & c_x c_y & c_x^2 & c_x c_z \\ -c_x c_z & -c_y c_z & -c_z^2 & c_x c_z & c_y c_z & c_z^2 \end{bmatrix}$$

where: $L = L^{(e)} = \sqrt{(x_j - x_i)^2 + (y_j - y_i)^2 + (z_j - z_i)^2}$

2-D Beam/Frame (NEGLECT axial deformation: 2 dof per node 4 dof per element)



$$[k_L^e] =$$

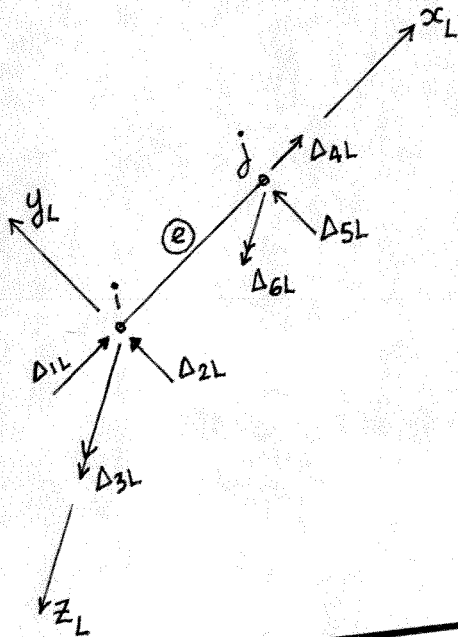
$$\begin{bmatrix} \frac{12EI_z}{L^3} & \frac{6EI_z}{L^2} & -\frac{12EI_z}{L^3} & \frac{6EI_z}{L^2} \\ \frac{6EI_z}{L^2} & \frac{4EI_z}{L} & -\frac{6EI_z}{L^2} & \frac{2EI_z}{L} \\ -\frac{12EI_z}{L^3} & -\frac{6EI_z}{L^2} & \frac{12EI_z}{L^3} & -\frac{6EI_z}{L^2} \\ \frac{6EI_z}{L^2} & \frac{2EI_z}{L} & -\frac{6EI_z}{L^2} & \frac{4EI_z}{L} \end{bmatrix}$$

where: $L = L^e = (x_j - x_i)$

$$[k_G^e] = [R]_{4 \times 4}^T [k_L^e]_{4 \times 4} [R]_{4 \times 4} = [k_L^e]$$

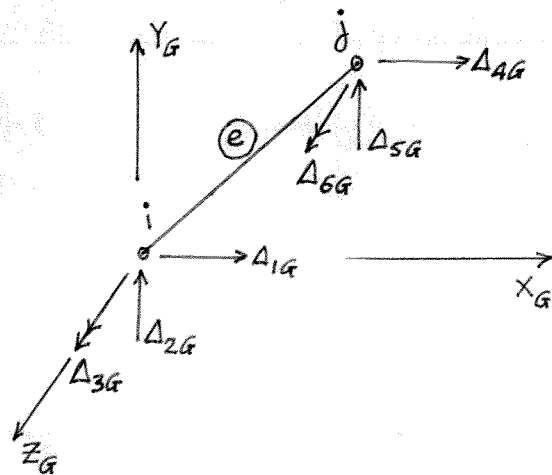
$[I]_{4 \times 4} = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix}$ since $x_L = X_G$
 $y_L = Y_G$

2-D Beam/Frame (INCLUDE axial deformation: 3 dof per node) 6 dof per element



$$[k_L^e] =$$

$$\begin{bmatrix} \frac{EA_x}{L} & 0 & 0 & -\frac{EA_x}{L} & 0 & 0 \\ 0 & \frac{12EI_z}{L^3} & \frac{6EI_z}{L^2} & 0 & -\frac{12EI_z}{L^3} & \frac{6EI_z}{L^2} \\ 0 & \frac{6EI_z}{L^2} & \frac{4EI_z}{L} & 0 & -\frac{6EI_z}{L^2} & \frac{2EI_z}{L} \\ \hline \frac{EA_x}{L} & 0 & 0 & \frac{EA_x}{L} & 0 & 0 \\ 0 & -\frac{12EI_z}{L^3} & -\frac{6EI_z}{L^2} & 0 & \frac{12EI_z}{L^3} & -\frac{6EI_z}{L^2} \\ 0 & \frac{6EI_z}{L^2} & \frac{2EI_z}{L} & 0 & -\frac{6EI_z}{L^2} & \frac{4EI_z}{L} \end{bmatrix}$$



$$[k_G^e] = [R]^T [k_L^e] [R]$$

Where: $[R] = \begin{bmatrix} [C] & [0] \\ [0] & [C] \end{bmatrix}$

$$[C] = \begin{bmatrix} c_x & c_y & 0 \\ -c_y & c_x & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$c_x = (y_j - y_i)/L^e$
 $c_y = (x_j - x_i)/L^e$

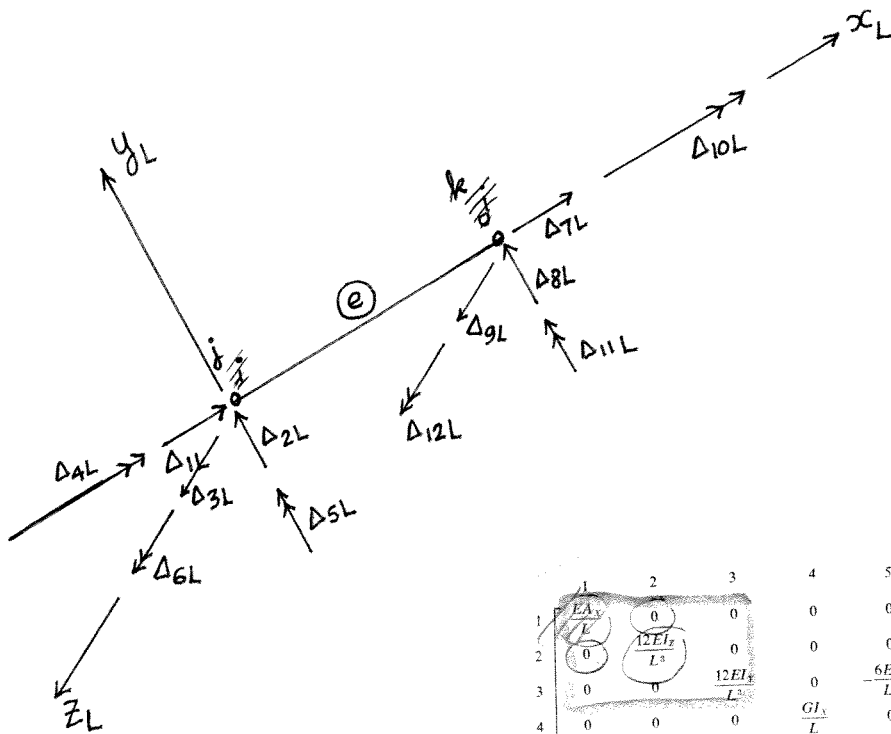
$$\text{and } L^e = \sqrt{(x_j - x_i)^2 + (y_j - y_i)^2}$$

Hence,

$$[k_G^e] =$$

$$\begin{bmatrix} \frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2 & \left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & -\frac{6EI_z}{L^2} C_y & -\left(\frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2\right) & -\left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & -\frac{6EI_z}{L^2} C_y \\ \left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & \frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2 & \frac{6EI_z}{L^2} C_x & -\left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & \frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2 & \frac{6EI_z}{L^2} C_x \\ -\frac{6EI_z}{L^2} C_y & \frac{6EI_z}{L^2} C_x & \frac{4EI_z}{L} & \frac{6EI_z}{L^2} C_y & -\frac{6EI_z}{L^2} C_x & \frac{2EI_z}{L} \\ \hline -\left(\frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2\right) & -\left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & \frac{6EI_z}{L^2} C_y & \frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2 & -\left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & -\frac{6EI_z}{L^2} C_y \\ -\left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & -\left(\frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2\right) & -\frac{6EI_z}{L^2} C_x & -\left(\frac{EA_x}{L} - \frac{12EI_z}{L^3}\right) C_x C_y & \frac{EA_x}{L} C_3^2 + \frac{12EI_z}{L^3} C_3^2 & -\frac{6EI_z}{L^2} C_x \\ -\frac{6EI_z}{L^2} C_y & \frac{6EI_z}{L^2} C_x & \frac{2EI_z}{L} & \frac{6EI_z}{L^2} C_y & -\frac{6EI_z}{L^2} C_x & \frac{4EI_z}{L} \end{bmatrix}$$

3-D Space Frame (6 dof per node, 12 dof per element)



$$[k_L^e]_{12 \times 12} =$$

	1	2	3	4	5	6	7	8	9	10	11	12
1	$\frac{EA_y}{L}$	0	0	0	0	0	$\frac{EA_y}{L}$	0	0	0	0	0
2	0	$\frac{12EI_y}{L^3}$	0	0	0	$\frac{6EI_y}{L^2}$	0	$\frac{12EI_y}{L^3}$	0	0	0	$\frac{6EI_y}{L^2}$
3	0	0	$\frac{12EI_z}{L^3}$	0	$\frac{6EI_z}{L^2}$	0	0	0	$\frac{12EI_z}{L^3}$	$\frac{GI_z}{L}$	0	0
4	0	0	0	$\frac{GI_z}{L}$	0	0	0	0	0	$\frac{GI_z}{L}$	0	0
5	0	0	$\frac{6EI_y}{L^2}$	0	$\frac{4EI_y}{L}$	0	0	0	$\frac{6EI_y}{L^2}$	0	$\frac{2EI_y}{L}$	0
6	0	$\frac{6EI_z}{L^2}$	0	0	0	$\frac{4EI_z}{L}$	0	$\frac{6EI_z}{L^2}$	0	0	0	$\frac{2EI_z}{L}$
7	$\frac{EA_y}{L}$	0	0	0	0	0	$\frac{EA_y}{L}$	0	0	0	0	0
8	0	$\frac{12EI_y}{L^3}$	0	0	0	$\frac{6EI_y}{L^2}$	0	$\frac{12EI_y}{L^3}$	0	0	0	$\frac{6EI_y}{L^2}$
9	0	0	$\frac{12EI_z}{L^3}$	0	$\frac{6EI_z}{L^2}$	0	0	0	$\frac{12EI_z}{L^3}$	$\frac{GI_z}{L}$	0	0
10	0	0	0	$\frac{GI_z}{L}$	0	0	0	0	0	$\frac{GI_z}{L}$	0	0
11	0	0	$\frac{6EI_y}{L^2}$	0	$\frac{2EI_y}{L}$	0	0	0	$\frac{6EI_y}{L^2}$	0	$\frac{4EI_y}{L}$	0
12	0	$\frac{6EI_z}{L^2}$	0	0	0	$\frac{2EI_z}{L}$	0	$\frac{6EI_z}{L^2}$	0	0	0	$\frac{4EI_z}{L}$

$$[k_a^e] = [R]^T [k_L^e] [R] \quad ; \quad \text{where: } [R] = \begin{bmatrix} [c] & [o] & [o] & [o] \\ [o] & [c] & [o] & [o] \\ [o] & [o] & [c] & [o] \\ [o] & [o] & [o] & [c] \end{bmatrix} \quad \dots (2.47)$$

$$[c] = \begin{bmatrix} c_x & c_y & c_z \\ \frac{-c_x c_y \cos(\alpha) - c_z \sin(\alpha)}{\sqrt{c_x^2 + c_z^2}} & (\sqrt{c_x^2 + c_z^2}) \cos(\alpha) & \frac{-c_y c_z \cos(\alpha) + c_x \sin(\alpha)}{\sqrt{c_x^2 + c_z^2}} \\ \frac{c_x c_y \sin(\alpha) - c_z \cos(\alpha)}{\sqrt{c_x^2 + c_z^2}} & (-\sqrt{c_x^2 + c_z^2}) \sin(\alpha) & \frac{c_y c_z \sin(\alpha) + c_x \cos(\alpha)}{\sqrt{c_x^2 + c_z^2}} \end{bmatrix} \quad \dots (2.48)$$

$$x_{ps} = x_p - x_j \quad ; \quad y_{ps} = y_p - y_j \quad ; \quad z_{ps} = z_p - z_j \quad \dots (2.50)$$

$$\sin(\alpha) = \frac{z_{ps}}{\sqrt{x_{ps}^2 + z_{ps}^2}} \quad ; \quad \cos(\alpha) = \frac{-x_{ps}}{\sqrt{x_{ps}^2 + z_{ps}^2}} * c_y \quad \dots (2.51)$$

and point "p" can be any point located on the principal plane cross-section [see Fig. 2.7]

Introduction to Stiffness Matrix Method

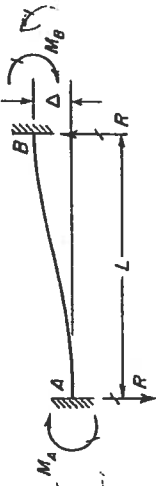
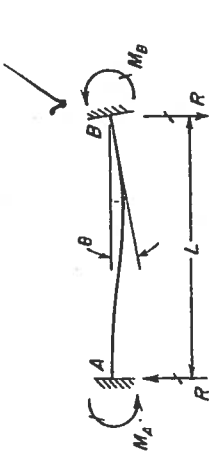
73

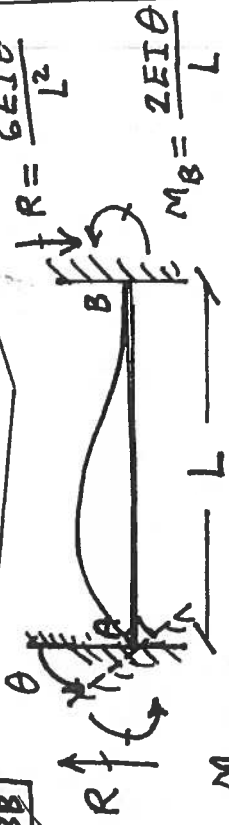
Table A.1

Fixed-End Actions Caused by Loads

1	2 $M_A = \frac{Mb}{L^2} (3a + b) \quad M_B = \frac{Ma}{L^2} (3b + a)$ $R_A = \frac{Mb}{L^3} (3a + b) \quad R_B = \frac{Ma}{L^3} (3b + a)$ $M_A = \frac{Mb}{L^2} (2a - b) \quad M_B = \frac{Ma}{L^2} (2b - a)$ $R_A = -R_B = \frac{6Mab}{L^3}$
3 $R_A = \frac{Pb}{L} \quad R_B = \frac{Pa}{L}$	4 $T_A = \frac{Tb}{L} \quad T_B = \frac{Ta}{L}$
5 $M_A = -M_B = \frac{Pa}{L} (L - a)$ $R_A = R_B = P$	6 $M_A = -M_B = \frac{wL^2}{12}$ $R_A = R_B = \frac{wL}{2}$
7 $M_A = \frac{wa^2}{12L^2} (6L^2 - 8aL + 3a^2)$ $M_B = -\frac{wa^2}{12L^2} (4L - 3a)$ $R_A = \frac{wa}{2L^3} (2L^3 - 2a^2L + a^3)$ $R_B = \frac{wa^3}{2L^3} (2L - a)$	8 $M_A = \frac{wL^2}{30} \quad M_B = -\frac{wL^2}{20}$ $R_A = \frac{3wL}{20} \quad R_B = \frac{7wL}{20}$

Fixed-End Actions Caused by End-Displacements

1	 $R = \frac{EA\Delta}{L}$
2A	 $M_A = M_B = \frac{6EI\Delta}{L^2} \quad R = \frac{12EI\Delta}{L^3}$
3A	 $M_A = \frac{2EI\theta}{L} \quad M_B = \frac{4EI\theta}{L} \quad R = \frac{6EI\theta}{L^2}$
4	 $T = \frac{GJ\phi}{L}$ G = shear modulus of elasticity J = torsion constant

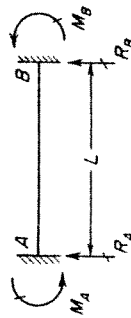
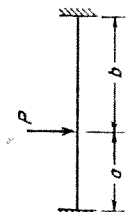
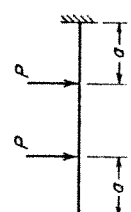
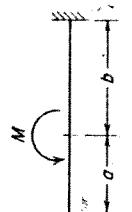
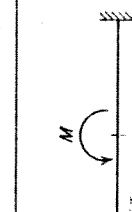
2B	 $M_A = M_B = \frac{6EI\Delta}{L^2}$ $R = \frac{12EI\Delta}{L^3}$
3B	 $M_A = \frac{4EI\theta}{L}$ $M_B = \frac{2EI\theta}{L}$ $R = \frac{6EI\theta}{L^2}$

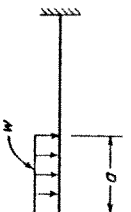
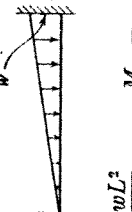
Introduction to Stiffness Matrix Method

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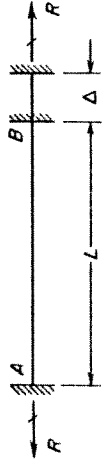
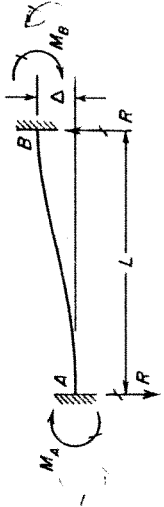
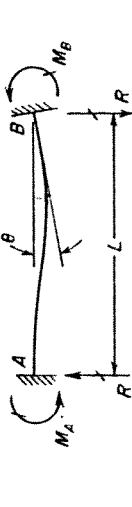
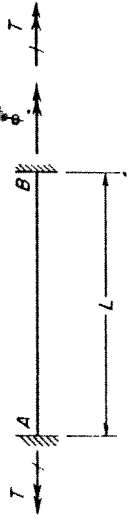
Table A.1

Fixed-End Actions Caused by Loads

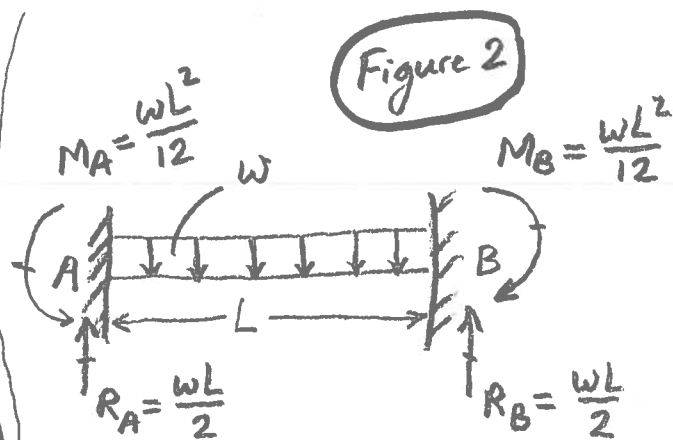
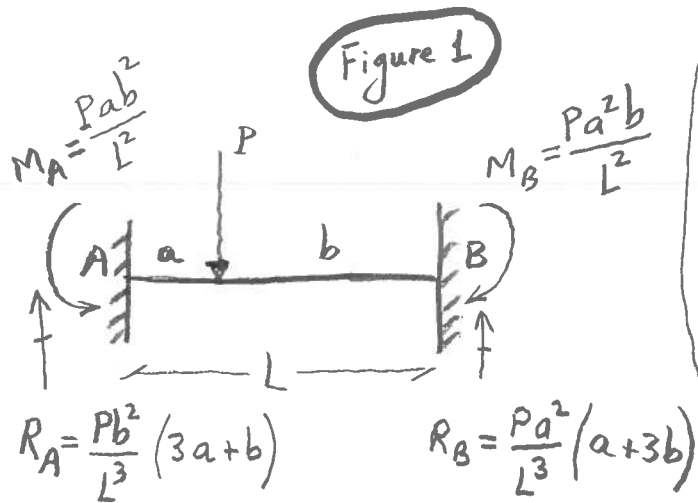
1 	2  $M_A = \frac{Pab^2}{L^2} \quad M_B = -\frac{Pa^2b}{L^2}$ $R_A = \frac{Pb^2}{L^3} (3a + b) \quad R_B = \frac{Pa^2}{L^3} (a + 3b)$	3  $M_A = -M_B = \frac{Pa}{L} (L - a)$ $R_A = R_B = P$
4  $M_A = \frac{Mb}{L^2} (2a - b)$ $M_B = \frac{Ma}{L^2} (2b - a) +$ $R_A = -R_B = \frac{6Mab}{L^3}$	5  $M_A = -M_B = \frac{wL^2}{12}$ $R_A = R_B = \frac{wL}{2}$	6  $M_A = \frac{wL^2}{30}$ $M_B = -\frac{7wL^2}{20}$ $R_A = \frac{3wL}{20}$ $R_B = \frac{7wL}{20}$

7  $M_A = \frac{wa^2}{12L^2} (6L^2 - 8aL + 3a^2)$ $M_B = -\frac{wa^3}{12L^2} (4L - 3a)$ $R_A = \frac{wa}{2L^3} (2L^3 - 2a^2L + a^3)$ $R_B = \frac{wa^3}{2L^3} (2L - a)$	8  $M_A = \frac{wL^2}{30}$ $M_B = -\frac{7wL^2}{20}$ $R_A = \frac{3wL}{20}$ $R_B = \frac{7wL}{20}$
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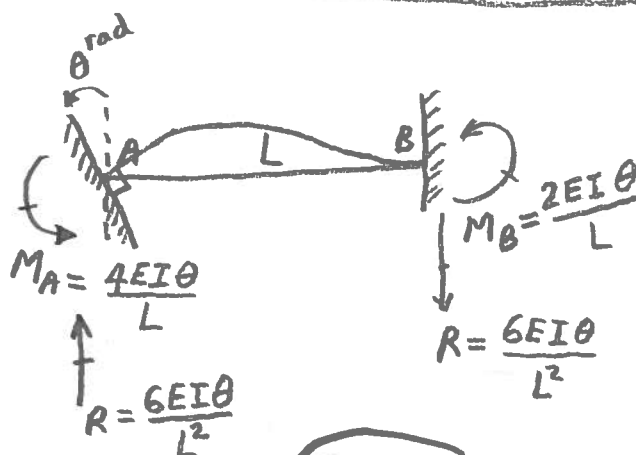
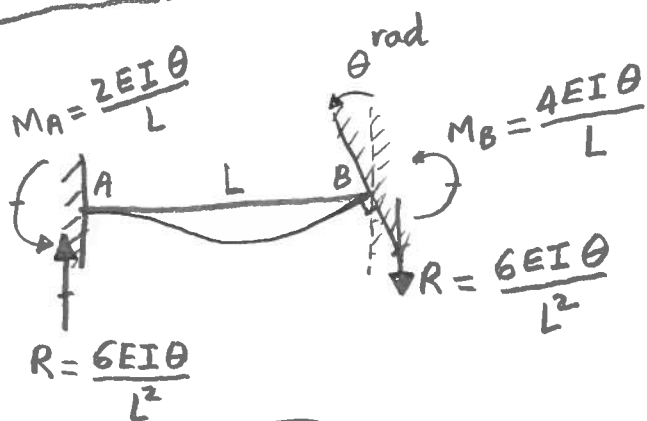
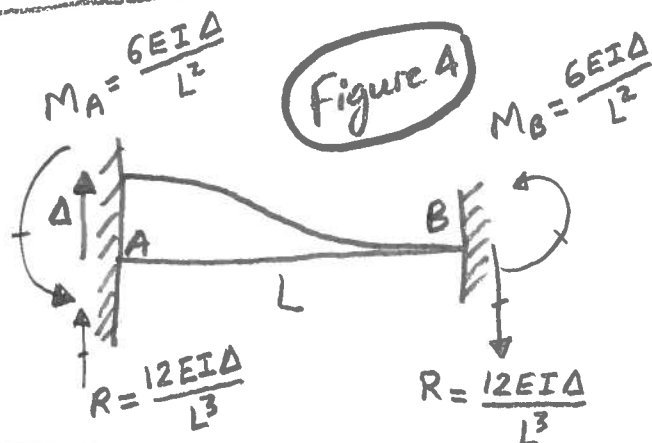
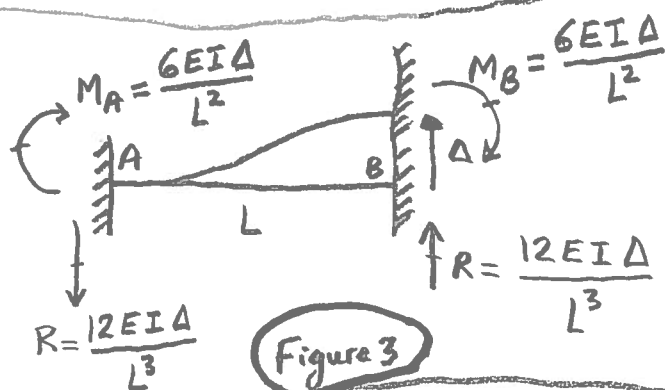
Fixed-End Actions Caused by End-Displacements

1	 $R = \frac{EA\Delta}{L}$
2A	 $M_A = M_B = \frac{6EI\Delta}{L^2} \quad R = \frac{12EI\Delta}{L^3}$
3A	 $M_A = \frac{2EI\theta}{L} \quad M_B = \frac{4EI\theta}{L} \quad R = \frac{6EI\theta}{L^2}$
4	 $T = \frac{GJ\phi}{L}$ G = shear modulus of elasticity J = torsion constant

2B	 $M_A = M_B = \frac{6EI\Delta}{L^2}$ $R = \frac{12EI\Delta}{L^3}$
3B	 $M_A = \frac{4EI\theta}{L}$ $M_B = \frac{2EI\theta}{L}$ $R = \frac{6EI\theta}{L^2}$



If $a = b = \frac{L}{2}$
 Then $|M_A| = |M_B| = \frac{PL}{8}$
 and $|R_A| = |R_B| = \frac{P}{2}$



Fixed End Actions Caused by Applied Loads & by Settlements

↙ ↘
 Moment Force

STIFFNESS METHOD

The following formulas are "always true/correct" for any element types

- 2-D/3-D Truss Element
- 2-D Beam/Frame Element
- 3-D Beam/Frame Element

$$[k_G^{(e)}] = [R]^T [k_L^{(e)}] [R] \quad (1)$$

where:

$$[R] = \begin{bmatrix} [C] & [0] \\ [0] & [C] \end{bmatrix} \quad (2)$$

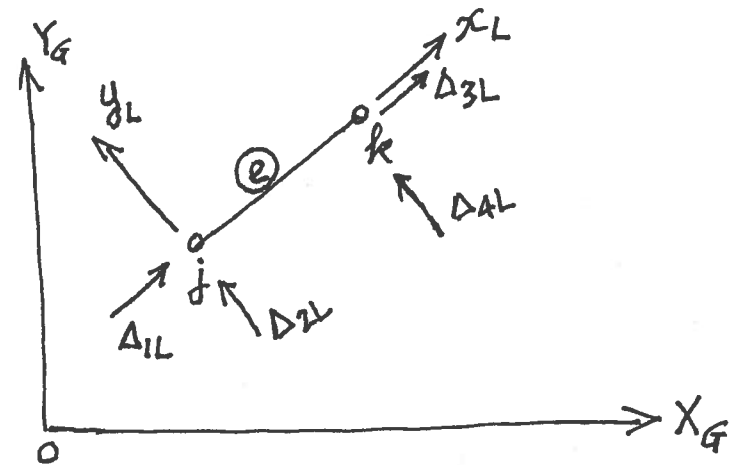
$$[C] = c_x, c_y, c_z, c_{xz}, \text{etc....} \quad (3)$$

$$c_x = \frac{x_k - x_j}{L_{jk}} \quad ; \quad c_{xz} = \sqrt{c_x^2 + c_z^2} \quad (4)$$

$$c_y = \frac{y_k - y_j}{L_{jk}} \quad (5)$$

$$c_z = \frac{z_k - z_j}{L_{jk}} \quad ; \quad L_{jk} = \sqrt{(x_k - x_j)^2 + (y_k - y_j)^2 + (z_k - z_j)^2} \quad (6)$$

2-D Truss



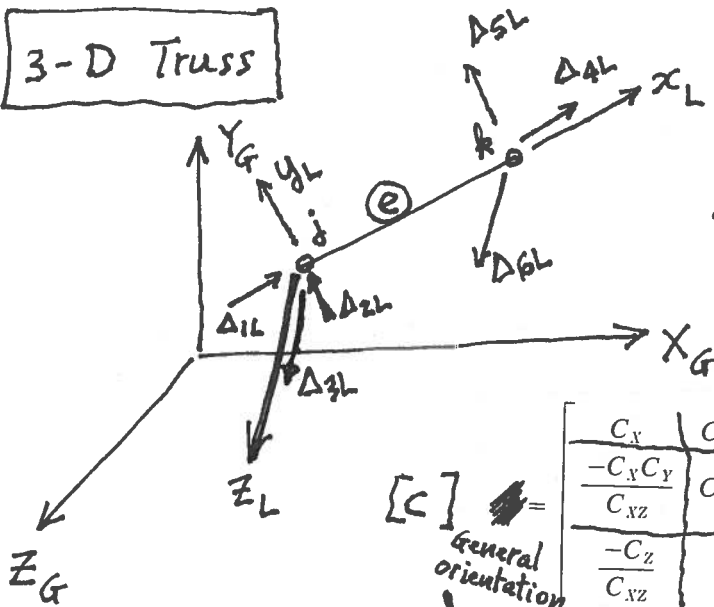
$$\frac{EA_A}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} = [k_L^e]$$

$$[C] = \begin{bmatrix} C_x & C_y \\ -C_y & C_x \end{bmatrix}$$

$$\frac{EA_A}{L} \begin{bmatrix} C_x^2 & C_x C_y & -C_x^2 & -C_x C_y \\ C_x C_y & C_y^2 & -C_x C_y & -C_y^2 \\ -C_x^2 & -C_x C_y & C_x^2 & C_x C_y \\ -C_x C_y & -C_y^2 & C_x C_y & C_y^2 \end{bmatrix}$$

$$[k_G^e] = [R]^T [k_L^e] [R]$$

3-D Truss



$$[k_L^e] = \frac{EA_A}{L}$$

$$\begin{bmatrix} 1 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -1 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[C] = \begin{bmatrix} C_x & C_y & C_z \\ -C_x C_y & C_{xz} & -C_y C_z \\ C_{xz} & 0 & C_{xz} \\ -C_z & 0 & C_{xz} \\ C_{xz} & 0 & C_{xz} \end{bmatrix}$$

General orientation

$$[C]_{\text{vertical member}} = \begin{bmatrix} 0 & C_y & 0 \\ -C_y & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

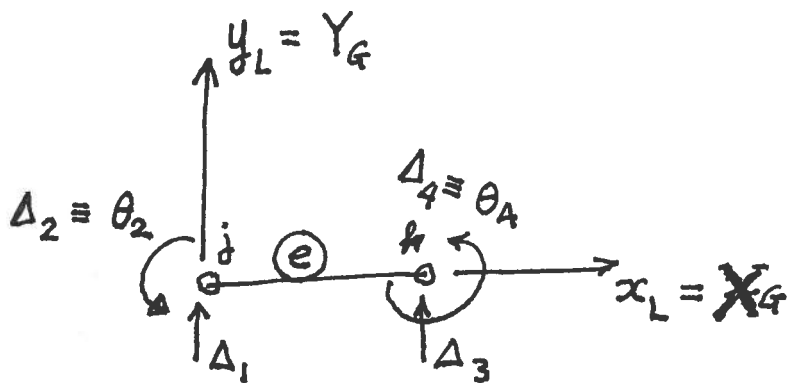
parallel to Y_G axis

$$[k_G^e] = \frac{EA_A}{L} \begin{bmatrix} C_x^2 & C_x C_y & C_z C_x & -C_x^2 & -C_y C_x & -C_z C_x \\ C_x C_y & C_y^2 & C_z C_y & -C_x C_y & -C_y^2 & -C_z C_y \\ C_x C_z & C_y C_z & C_z^2 & -C_x C_z & -C_y C_z & -C_z^2 \\ -C_x^2 & -C_x C_y & -C_x C_z & C_x^2 & C_x C_y & C_x C_z \\ -C_y C_x & -C_y^2 & -C_y C_z & C_x C_y & C_y^2 & C_y C_z \\ -C_z C_x & -C_z C_y & -C_z^2 & C_x C_z & C_y C_z & C_z^2 \end{bmatrix}$$

$$= [R]^T [k_L^e] [R]$$

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2-D Beam (Neglect Axial)

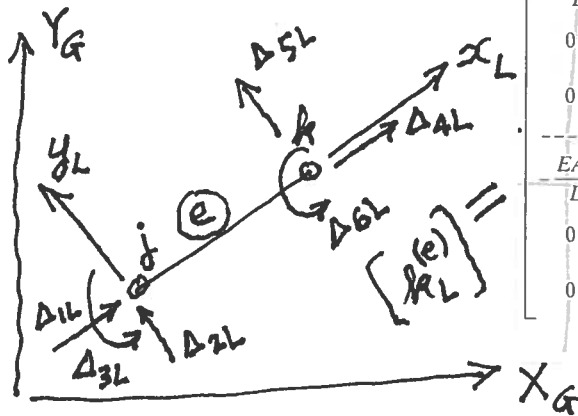


$$[k^e] = \begin{bmatrix} \frac{12EI_z}{L^3} & \frac{6EI_z}{L^2} & -\frac{12EI_z}{L^3} & -\frac{6EI_z}{L^2} \\ \frac{6EI_z}{L^2} & \frac{4EI_z}{L} & -\frac{6EI_z}{L^2} & -\frac{2EI_z}{L} \\ -\frac{12EI_z}{L^3} & -\frac{6EI_z}{L^2} & \frac{12EI_z}{L^3} & \frac{6EI_z}{L^2} \\ \frac{6EI_z}{L^2} & \frac{2EI_z}{L} & -\frac{6EI_z}{L^2} & \frac{4EI_z}{L} \end{bmatrix}$$

$$= [k_G^e] = [R]^T [k_L^e] [R]$$

$$[I] = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix}$$

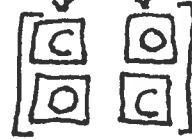
2-D Beam/Frame (Include Axial)



$\frac{EA_Y}{L}$	0	0	$-\frac{EA_Y}{L}$	0	0
0	$\frac{12EI_Z}{L^3}$	$\frac{6EI_Z}{L^2}$	0	$-\frac{12EI_Z}{L^3}$	$\frac{6EI_Z}{L^2}$
0	$\frac{6EI_Z}{L^2}$	$\frac{4EI_Z}{L}$	0	$-\frac{6EI_Z}{L^2}$	$\frac{2EI_Z}{L}$
$\frac{EA_Y}{L}$	0	0	$-\frac{EA_Y}{L}$	0	0
0	$-\frac{12EI_Z}{L^3}$	$-\frac{6EI_Z}{L^2}$	0	$\frac{12EI_Z}{L^3}$	$-\frac{6EI_Z}{L^2}$
0	$\frac{6EI_Z}{L^2}$	$\frac{2EI_Z}{L}$	0	$-\frac{6EI_Z}{L^2}$	$\frac{4EI_Z}{L}$

$$\begin{bmatrix} C_X & C_Y & 0 \\ -C_Y & C_X & 0 \\ 0 & 0 & 1 \end{bmatrix} = [C]$$

3x3 sub-matrices



$$\begin{bmatrix} k_G^e \end{bmatrix}_{6 \times 6} = \begin{bmatrix} R \end{bmatrix}_{6 \times 6}^T \begin{bmatrix} k_L^e \end{bmatrix}_{6 \times 6} \begin{bmatrix} R \end{bmatrix}_{6 \times 6}$$

$[k_G^e] =$

$$\begin{bmatrix} \frac{EA_Y}{L} C_X^2 + \frac{12EI_Z}{L^3} C_Y^2 & \left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & -\frac{6EI_Z}{L^2} C_Y & -\left(\frac{EA_Y}{L} C_X^2 + \frac{12EI_Z}{L^3} C_Y^2\right) & -\left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & -\frac{6EI_Z}{L^2} C_Y \\ \left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & \frac{EA_Y}{L} C_Y^2 + \frac{12EI_Z}{L^3} C_X^2 & \frac{6EI_Z}{L^2} C_X & -\left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & -\left(\frac{EA_Y}{L} C_Y^2 + \frac{12EI_Z}{L^3} C_X^2\right) & \frac{6EI_Z}{L^2} C_X \\ -\frac{6EI_Z}{L^2} C_Y & \frac{6EI_Z}{L^2} C_X & \frac{4EI_Z}{L} & \frac{6EI_Z}{L^2} C_Y & -\frac{6EI_Z}{L^2} C_X & \frac{2EI_Z}{L} \\ -\left(\frac{EA_Y}{L} C_X^2 + \frac{12EI_Z}{L^3} C_Y^2\right) & -\left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & \frac{6EI_Z}{L^2} C_Y & \frac{EA_Y}{L} C_X^2 + \frac{12EI_Z}{L^3} C_Y^2 & \left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & \frac{6EI_Z}{L^2} C_Y \\ -\left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & -\left(\frac{EA_Y}{L} C_Y^2 + \frac{12EI_Z}{L^3} C_X^2\right) & -\frac{6EI_Z}{L^2} C_X & \left(\frac{EA_Y}{L} - \frac{12EI_Z}{L^3}\right) C_X C_Y & \frac{EA_Y}{L} C_Y^2 + \frac{12EI_Z}{L^3} C_X^2 & -\frac{6EI_Z}{L^2} C_X \\ -\frac{6EI_Z}{L^2} C_Y & \frac{6EI_Z}{L^2} C_X & \frac{2EI_Z}{L} & \frac{6EI_Z}{L^2} C_Y & -\frac{6EI_Z}{L^2} C_X & \frac{4EI_Z}{L} \end{bmatrix}$$

"System" Equations

Contributions from

"Element" stiffness equations!!

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{14} & K_{15} & K_{16} \\ K_{21} & K_{22} & K_{23} & K_{24} & K_{25} & K_{26} \\ K_{31} & K_{32} & K_{33} & K_{34} & K_{35} & K_{36} \\ K_{41} & K_{42} & K_{43} & K_{44} & K_{45} & K_{46} \\ K_{51} & K_{52} & K_{53} & K_{54} & K_{55} & K_{56} \\ K_{61} & K_{62} & K_{63} & K_{64} & K_{65} & K_{66} \end{bmatrix} \begin{Bmatrix} \Delta_1? \\ \Delta_2? \\ \Delta_3? \\ \Delta_4 = C_4 = 0 \\ \Delta_5 = C_5 = 0 \\ \Delta_6 = C_6 = 0 \end{Bmatrix} = \begin{Bmatrix} 10^K \\ 0^K \\ 0^K \\ R_{2y}? \\ R_{3x}? \\ R_{3y}? \end{Bmatrix} \dots (2.56)$$

"singular" matrix

mixing knowns & unknowns

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} & 0 & 0 & 0 \\ K_{21} & K_{22} & K_{23} & 0 & 0 & 0 \\ K_{31} & K_{32} & K_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} \Delta_1 \\ \Delta_2 \\ \Delta_3 \\ \Delta_4 \\ \Delta_5 \\ \Delta_6 \end{Bmatrix} = \begin{Bmatrix} 10^K - K_{14}C_4 - K_{15}C_5 - K_{16}C_6 \\ 0^K - K_{24}C_4 - K_{25}C_5 - K_{26}C_6 \\ 0^K - K_{34}C_4 - K_{35}C_5 - K_{36}C_6 \\ C_4 \\ C_5 \\ C_6 \end{Bmatrix} \dots (2.60)$$

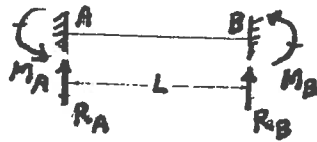
"Non-singular"

RHS vector contains all known quantities

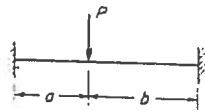
- Using Eq. (2.60) to solve $\Delta_1, \Delta_2, \dots, \Delta_6$
- Then, using Eq. (2.56) to solve all unknown support reactions, such as R_{2y}, R_{3x} & R_{3y}

<http://www.lions.odu.edu/~skadi002>
Then SELECT/CLICK Educational Assessment Tools
Then select/click Simultaneous Linear Equations (SLE)

Fixed-End Actions Caused by Loads



1



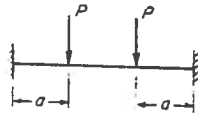
If $a = b = \frac{L}{2}$

$$R_A = R_B = \frac{P}{2}$$

$$M_A = \frac{Pab^2}{L^2} \quad M_B = -\frac{Pa^2b}{L^2}$$

$$R_A = \frac{Pb^2}{L^3} (3a + b) \quad R_B = \frac{Pa^2}{L^3} (a + 3b)$$

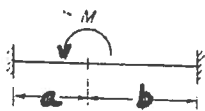
5



$$M_A = -M_B = \frac{Pa}{L} (L - a)$$

$$R_A = R_B = P$$

2

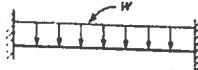


$$M_A = \frac{Mb}{L^2} (2a - b)$$

$$M_B = \frac{Ma}{L^2} (2b - a)$$

$$R_A = -R_B = \frac{6Mab}{L^3}$$

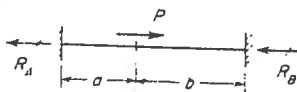
6



$$M_A = -M_B = \frac{wL^2}{12}$$

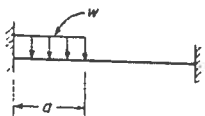
$$R_A = R_B = \frac{wL}{2}$$

3



$$R_A = \frac{Pb}{L} \quad R_B = \frac{Pa}{L}$$

7



$$M_A = \frac{wa^2}{12L^2} (6L^2 - 8aL + 3a^2)$$

$$M_B = -\frac{wa^3}{12L^2} (4L - 3a)$$

$$R_A = \frac{wa}{2L^3} (2L^3 - 2a^2L + a^3)$$

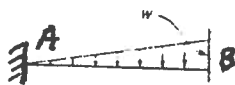
$$R_B = \frac{wa^3}{2L^3} (2L - a)$$

4



$$T_A = \frac{Tb}{L} \quad T_B = \frac{Ta}{L}$$

8

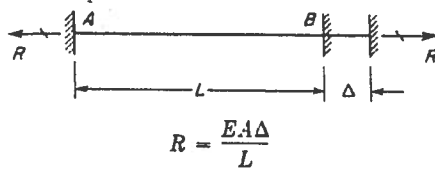


$$M_A = \frac{wL^2}{30} \quad M_B = -\frac{wL^2}{20}$$

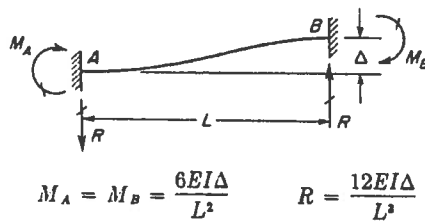
$$R_A = \frac{3wL}{20} \quad R_B = \frac{7wL}{20}$$

Fixed-End Actions Caused by End-Displacements

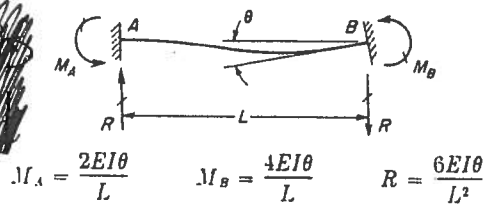
1



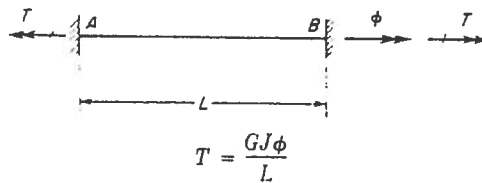
2



3

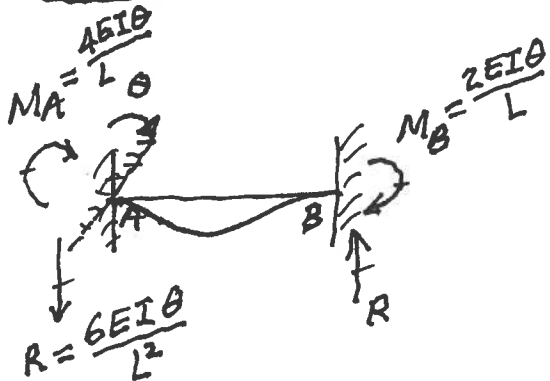
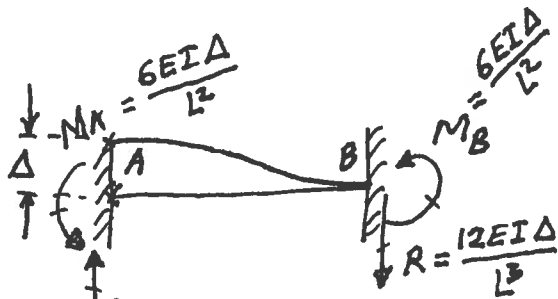


4



G = shear modulus of elasticity

J = torsion constant



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